Government of Madras.

29/91/56 Public (1955) Department.

Proceedings 151.RDP.UI/55-1

dated 20/11/55.

Passed

Sd/-

20/12/55

Ref.C.No.3297/53-B.

Project Executive Office, S.K.,
Mangalore, dated 30--12--1954.

From
Sri. H. Narasimha,
Project Executive Officer,
South Kanara, MANGALORE.

To
The Director of Community
Development, MADRAS.

ASSISTANT SECRETARY TO GOVERNMENT
5 JAN 1955
151
(RURAL DEVELOPMENT PROJECTS), PUBLIC DEPT.

Sir,

Sub:- COMMUNITY PROJECT--South Kanara--Block II--Venur-Murjee.
Road Construction of a bridge--Administrative--Sanction--Requested.
--

I submit herewith an estimate in duplicate with plans etc. for the construction of a bridge on the road from Venur to Murjee in Block II. The particulars required are furnished below:

1. Block:	:	II
2. Name of work:	:	Construction of a bridge on Venur Murjee road.
3. Whether the work is approved and included in Overall Programme.	) :)	Yes. It corresponds to item No.49 in Block II under Katcha roads.
4. Estimated cost:	:	1. Road work & paved dips etc. : Rs. 12,900/-. 2. Bridge: Rs. 25,000/-. Total. Rs.37,900/-
5. Villagers contribution:		1. Road work & paved dip etc.: Rs.12,900/-
6. Expenditure to be borne by Community Project:		Rs.25,000/- for Bridge work.

Remarks: This work relates for the formation of a road about 5½ miles, construction of dips and construction of a bridge on the road from Venur to Murjee. This road will connect Venur with Buntwal and pass through a good number of interior and backward villages lying in the Malnad area where lack of communication facilities is the prime reason for their undeveloped condition. The road work and construction of dips etc. at an estimated cost of Rs. 12,900/- will be done by the villagers as their contribution towards the entire work.

The estimated cost of the bridge is Rs. 25,000/- and the same has been technically sanctioned by the Divisional Engineer (D), Community Project, Mangalore.

I request that administrative sanction to the estimate for the construction of the bridge for Rs. 25,000/- may be accorded and the estimate returned for taking necessary further action.

Connected file of estimate & plans is herewith enclosed.

As the working season is short, the Spl. Divisional Engineer (C.P.) proposes to take steps to start the work in anticipation of sanction. I have agreed to his proposal in view of the urgency of the work.

Yours faithfully,

31.12.54

Project Executive Officer.

JC 1-72-54.

P.W.D. V–39—57,603 –4–4–51.

Data sheet to accompany the estimate for Bridge at —

DIVISIONAL ENGINEER'S OFFICE
– 7 JUIN 1954

Quantity.	Description of work.	Rate.	Per	Amount.	Total.
	Carried forward	RS. A. P.		RS. A. P.	
100 Cft	Rubble masonry in foundations	21 6	100 Cft	21 6	
	Add 100 % increase			21 6	
				42 12	/ 100 Cft
	Excavating in gravelly soil for foundations	1 4	1 Cft	1 4	/ Cft
2 Cft	Cement mortar 1:2 for 2 Courses	1 4 0	Cft	2 8 0	3-12-0 2-8-0
100 Cft	Sand	2 8 -	100 Cft	2 8 -	
100 Cft	Lead for do from Quarry	5 8 -	"	5 8 -	
33⅓ Cft	Cement	8 8 -	Ton	8 8 -	
				314 10 8	
Do.	Lead REMAIN 3 MCT	70 15 8	100 Cft	17 11 11	
				23 10 7	
100 Cft	Mixing Charges Sundries	4 8 -	"	4 8 0	
			Ls.	- 6 8	129/- / 100 Cft for 1-11-80
	Carried forward			118 - -	
4)	Concrete Rock from stone in Cement mortar 1:2:6.				
92 Cft	metal 1½" size	23 - -	100 Cft	21 2 7	
46 Cft	mortar as above	171 8 6	"	7 6	59-6-7 78-14-3
½	Mason 2nd Class	2 12 -	each	1 6 -	
5	Man Mazdoor.	1 8 -	"	7 8 -	
4	Woman "	0 13 -	"	3 4 -	
92 Cft	Lead ½ MCT. Sundries	5 13 9	100 Cft	5 6 3	0-6-11 98/4/-
			Ls.	- 3 9	/ 100 Cft = 118/-
				93 - -	
				98 4 -	
5)	Coursed rubble masonry in 2nd Sort			C.M 1:5	
100 Cft	Coursed rubble stone including bond stones.	28 - -	100 Cft	15 12 9	
50 Cft	Rough stones	16 - -	"	8 - -	
82 Cft	mortar as above	106 - -	"	33 14 9	
7	Mason 2nd Class	2 12 -	each	19 4 -	
4	Man Mazdoor.	1 8 -	"	6 - -	
4	Woman "	0 13 -	"	3 4 -	
100 Cft	Lead as above Sundries	5 13 9	100 Cft	6 7 5	94-0-0
			Ls.	5	/ 100 Cft for 100 Cft.
	Coursed rubble masonry in 1st Sort			C.M 1:5	
				94 0 0	
115 Cft	stone including bond stones	38 - -	100 Cft	41 12 10	
28 Cft	mortar as above	186 - -	"	33 0 8	
10	Mason 2nd Class	2 12 -	each	27 8 -	
4	Man Mazdoor.	1 8 -	"	6 - -	
4	Woman "	0 13 -	"	3 4 -	
115 Cft	Lead as above	5 13 9	100 Cft	6 7 1	
	Sundries		Ls.	2	115-0-0
				115 0 0	/ 100 Cft for 100 Cft

P.T.O.

P.W.D. V–39—57,603—4-4-51.

Data sheet to accompany the estimate for_____

Quantity.	Description of work.	Rate. Rs.	A.	P.	Per	Amount. Rs.	A.	P.	Total.
	Cement mortar 1:2								
	Sand								
	Lead								
	Cement								
	Lead as above								
	Mixing Charges								
	Cement Concrete 1:2:4								
	Lead								
	Lead as above								
	Centering for Arch								
	Concrete								
	Lead								
	Centering for Arch								
	Cement Concrete 1:2:4								
	Concrete								
	Lead								
	Centering for Arch								

P.W.D. V–39—57,603—4-4-51.

Data sheet to accompany the estimate for_____

DIVISIONAL ENGINEER'S OFFICE
Rec'd
- 7 JUN 1954

Quantity.	Description of work.	Rate.	Per	Amount.	Total.
		Rs. A. P.		Rs. A. P.	
	12) Random stone-dry pitching for protection work.				
11 Cft	for Random stone	1 0 0	1 cft	11 0 0	
1 No	Hand dressing	3 0 0	each	3 0 0	
3 Cft	Do on face	1 0 0	"	1 8 0	
2 Cft	Do on joints	0 8 0	"	1 0 0	
				0 8 0	
11 Cft	Lead as above	5 13 9	100 cft	0 11 4	
	1 Contingencies			0 1 4	
				3 0 0	
				32 4 0	
	13) Random stone pitching as above but with joints pointed with cement mortar 1:3				
	Lead & lift				
11 Cft	as 31 (a) B.	1 10 0	1 cft	18 10 0	
	Certified that the rates and heads entered are correct to the best of my knowledge				
	7.2.55				
	Inspection Officer				
	Cement Mortar 1:5				
100 cft	Sand	2 8 0	100 cft	2 8 0	
100 cft	Lead for sand as above	5 9 0	"	5 9 0	
20	Cements	98 8 0	285	78 12 6	
do	Lead as above	70 1 8	100 cft	14 3 2	
100 cft	Mixing for mortar	4 8 0	100 cft	4 8 0	
	Sundries			7 1 0	
				106 0 0	
~~60 cft~~	~~Coursed Rubble masonry in C.M. 1:5~~				2nd part
60 cft	Coursed Rubble	28 0 0	100 cft	16 12 8	
~~50 cft~~	~~Ramp~~				

P.W.D. V-39—57,603—4-4-31.

Data sheet to accompany the estimate for_____

Quantity.	Description of work.	Rate. Rs.	A.	P.	Per	Amount. Rs.	A.	P.	Total.
	~~Work Contd~~								
	Concrete Land breaker stone in cement mortar 1:1½:3 mix using ½" & ¾" broken metal and 1" size metal 66⅔								
30.66 yds	Broken metal ½" size	53	.	.	100	16	10	4	
61.33 yds	Broken metal 1" size	30	.	.	100	18	6	5	
92 yds	Broken stone head	6	14	.	100	6	5	2	
46 yds	Land cord	2	8	.	100	1	2	5	
30.66	Cement Cord	98	8	.	100	120	13	.	
30.66	Cement & Land	70	15	8	100	21	12	2	
46 sft	Granite moulding	¼	8	.	100	2	1	1	
1 No	Granite 2nd class	2	12	.	each	2	12	.	
1 No	Granite mag door	1	8	.	each	9	.	.	
10 Nos	Granite mag door	0	13	.	each	8	2	.	
L.S.	Sundries					0	13	5	
						208	0	.	208-00

(3)

Specification Report to accompany the estimate for Constructing a bridge across Hosapatna River in Vehur Murjee Road General:

At present there is no proper communication connecting these two villages. It is therefore proposed to form a road connecting these two villages. The road will be formed by the villagers ~~and~~ 24'. The road passes thro' Hosapatna where there is a Stream crossing and it is proposed to construct a bridge across this Stream. The bridge will be constructed by the Community Project Department. The report ~~comprising~~ financial aspect for the construction of the bridge will be made by the A.P.O Kundapur

Site: The bridge is proposed to be constructed at the site where the road formed by the villagers stops at the banks. The banks at this site are well defined.

Type: The width of the stream at the site is 90'-0". It is found that floating timber is encountered during M.F.L Conditions. Hence it is proposed to adopt large spans so as to afford least obstruction to the floating timber. The discharge has been worked out adopting both the Rynos ~~and~~ A formulae and Area ~~cross section~~ Velocity Method. The value of C has been taken as 2000 since it is a hilly Stream ~~suggested to be a mountain~~. The discharge obtained by Area velocity method is the maximum and hence it is adopted for waterway Calculation. From the hydraulic Calculations it is found that 3 spans of 30'-0" each are quite sufficient to discharge the flood water.

6

Foundation: Trial probings taken during investigation indicated that sound rock is available at a depth of 1½' to 2' below bed level. The sound rock will be properly benched during excavation and a levelling course of cement concrete 1:3:6 mix 0'-6" thick will be laid over it. using 1" dia dowel bars per sq.m. anchored against sliding.

Sub-structure: The abutments and wings are proposed to be built with CR masonry II sort in CM 1:5. The top width of abutment will be 3'-0" and bottom width (0.4H+1'). H front batter of 1 in 12 is provided for abutments. The piers will be built of CR masonry I sort in CM 1:5. The top width of pier will be 2'-6" and suitable footing has been provided as indicated in the detailed drawings.

Superstructure: The superstructure will be of RCC 1:2:4 slab 25" thick with clear spans of 30'-0" each designed for IRC B class loading as per instruction of the Divisional Engineer (H) Mangalore. The clear width of roadway will be 12'-0". The wearing surface will consist of cement concrete (1:1½:3), 3" av. thick with 66⅔ % of 1" dia grit and 33⅓ % of ¾" dia stone laid over the RC check slab, the top being properly ~~levelled~~ Camber to slope towards the edges for drainage purpose. Provision has been made for wheel guards of 0'-9" x 0'-11" and RC Posts of 0'-5" x 0'-7" and hand rails 0'-5" x 0'-6"

A lumpsum provision has been made for the formation of approach road, leading out water during excavation and construction of temporary sheds to store materials etc.

The whole work will be carried out as per the MDSS and other relevant circulars.

K Maxwell 27.11
Divisional Engineer (H)
Community Projects,
Mangalore.

H S Shetty
12/12/84
Junior Engineer (P)
Sub Unit Karkala-Ayanna

Approved
Asst Engr
13/12

Hydraulic Calculations for Hosapatna Bridge

Area of cross section $A = 1060$ Sq.ft.

Wetted Perimeter $P = 160'0"$

Hydraulic mean depth $r = \frac{A}{P} = \frac{1060}{160} = 6.6.$

Bed slope 'S' $= \frac{87.77 - 87.18}{200} = .003.$

Adopting Manning's formula $q_c = \frac{1.486 r^{2/3} S^{1/2}}{.035}$

$q_c = \frac{1.486 \times 6.6^{2/3} \times .003^{1/2}}{.035} = 8'/\text{sec}.$

$\therefore$ Discharge $= A \times q_c$

$= 1060 \times 8$

$= 8480$ cusecs.

Catchment Area - 8 Sq. miles

Adopting Ryve's formula $CM^{2/3}$
and assuming the value of C as 2000

Discharge $= 2000 \times 8^{2/3}$

$= 8000$ cusecs

Hence the max discharge as obtained by Area velocity method is adopted for waterway calculation.

M.F.L $= 100'.00$

Average bed level $= 88.00$

$\therefore$ Depth of flow $= 12.00$

Adopting a linear waterway of $90'-0"$ being the distance from bank to bank velocity throughout

$\frac{8480}{90 \times 12} = 7.9'/\text{sec}.$

This is permissible.

Hence adopt 3 spans of $30'0"$ each.

10/Bottom width of Aqueduct.

Road Level = 105.33.

Sill Level = 86.5

Height 18.83.

Bottom width = .4 L + 1

= 18.83 × .4 + 1

= 8' 6".

Checked
12/12

Executive Engineer
9/12

Divisional Engineer (HQ)
Community Projects,
Mangalore.

Design of deck slab.

clear Span 30'-0"

Effective Span = 30 + 2·08 = 32·08'.

Impact factor = $\frac{15}{20 + 32.08} = 0.3$

Intensity of load per foot width of slab

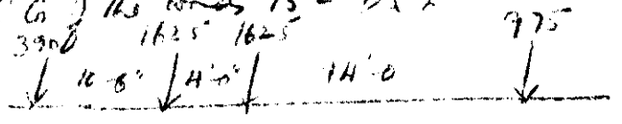
15000 lb load = $\frac{15000 \times 1.3}{12} = 1625$ lb

9000 lb load = $\frac{9000 \times 1.3}{12} = 975$ lb

3600 lb load = $\frac{3600 \times 1.3}{12} = 390$ lb.

For max BM the centre of span should be equidistant to the C.G. of the loads carried and the load nearest which the max BM occurs.

The C.G. of the loads B.O.D.F

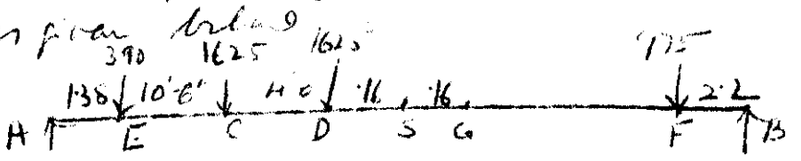


$$\frac{1625\,(10'6 + 14'6') + 975 \times 28'6}{4615}$$

= 14·82 from pt B.

The comparison of the load & CG therefore for

max final load



$R_B \times 32·08 = 390 \times 1·35 + 1625\,(11·58 + 15·88) + 975 \times 1·88$

$R_B = 2331$ lb.

Max BM at D $= (2331 \times 16·2 - 975 \times 14) 12$

= 2,89,440 in lb

Dead load.

Deck Slab $= \frac{25}{12} \times 150 = 313$

Wearing Coat $= \frac{3}{12} \times 140 = \frac{35}{348}$ or 350 lb.

B.M due to dead load $= \frac{350 \times 32·08 \times 32·08 \times 12}{8}$

= 5,40,300 in lbs.

Total BM = 8,29,740 in lb.

12

Resisting moment = 125.74 bd2

$$d = \sqrt{\frac{8,29,740}{125.74 \times 12}}$$

$$= 23.46"$$

$A_t = .008\ bd$

$= .008 \times 12 \times 23.46$

$= 2.25\ sq"$

Hence, provide 1"ϕ bars at 4" c/c which give an area of 2.36 sq"

Overall depth $= 23.46 + 1 + 0.5 = 24.96$ sq

$= 25"$

To resist negative BM at supports additional bars may be curtailed at 0.15ℓ or 4'-6" from supports

Distributors $= \frac{1}{\sqrt{320}} \times 2.25 = 0.41$ sq"

Hence provide ½" ϕ bars at 5½" c/c which gives an area of 0.41 sq".

Quantity of steel per sft of slab = 5.67 lbs.

Assistant Engineer CP
Sri Ramanagara

Dhanuja
27.12.

Asst. Engineer (H)
Community Projects,
Mangalore.

Executive Engineer
Proj. Div. 3/12

Construction Bridge at Mangakahia in Vernon Murgin Road

Description	nos	Length	Breadth	Depth	Area or Contents	Total
1. Earthwork in all soils except rock lying below 66 ft	1·5				10000 yds	
2 Benching rock in 2 layers 9" deep	1·5				500 sq yds	
3. Cement Concrete 1:3:6 ~~1:4:8~~ with 1½" Road Broken stone						
Left Abutment	1	18'0"	9'6" ~~10'6~~	1'0" ~~0'6~~ 1'0"	~~171~~ 86	~~185~~ 171
Wings	1×2	9'0"	1'0"	~~0'6~~ 1'0"	~~9 18~~	18
Right Abutment	1	18'0"	8'6"	~~0'6~~ 1'0"	~~77~~	153
Wings	2×1	9'0"	1'9" 4'0"	~~0'6~~ 1'0"	~~16~~	32
Piers I & II	2	17'6"	~~3'6~~	~~0'6~~ 1'0"	~~60~~	140
~~130 cubic ft of brickwork in foundations~~					~~5 1~~	~~115 yds~~ 525 cft
4. Concrete Rubble Masonry in Piers II & III Portion B. Rectangular Portion A Quadrant C.	1	14'0"	1'6"+3'0" 2	17'0"	536	
	2	22/7 × (1·5)² × 1/4		17'0"	60	
Left Abutment 8'6" to 10'3"	1	17'0"	~~8'6"+11'~~ 2	17'0"	1012 ~~166 2~~	
Wings 8'6" to 10'3"	2	5'6"+1'9" 2	2'0"+8'6" 2	17'0"	536	
Right Abutment 1 Portion A 8'6" to 10'3"	1	17'0"	~~11'6"+11'6"~~ 2	14'0"	744 ~~250~~	
Wings 8'6" to 10'3"	2	5'0"+1'9" 2	2'0"+7'6" 2	14'0"	447 414	
Wings Right ~~Left~~ & Left 10'3" to 10'5·33"	2×2	5'6"	1'6"	2·33	70	
Backing Concrete	2	14'0"	1'6"	2·08	87	
Reinforced Concrete Block	2	13'6"	1'6"	1'0"	(-)41	
Add for Right Abutment Portion B rectangular portion 1 Quadrant 2	1	14'0"	1'6"+2'9" 2	14'0"	417	
	2	22/7 × (1·5)² × 2/4		14'0"	50	4005 yds or 4100 cft
Piers I & II 8'6" to 10'3"	1		3'0	9'0"	365	3920 or 3950 cft
Cutwater	2 ×	22/7 ×	~~3'0~~ 2	~~×3'0"~~ × 1/2	×9'0"	64
Pier II 8'6" to 10'3"	1	15'6"	3'0"	6'0"	324	
Cutwater	2	22/7 ×	2'0"×3'0" 4	× 1/2 ×	6'0"	36
Pier I 8'6" to 10'3"	2	13'6"	2'6"	6'0"	1105	
Cutwater	2×2	22/7 ×	~~2'0"~~ 4	× 1/2	×6'	54
Pier III	2×1	16'	16'	3'0"	54	
Cutwater	2×2	3'6"	1'5"	2'6"	111	1271 or 1300

14

Description	No	Length	Breadth	Depth	Cu. Ft.	Total
6 Concrete 1:2:4 with 3/4" hard broken stone						
For Deckslab Abutment & Pier	2	13'6"	1'6"	1'6"	41	
	2	13'6"	2'0"	1'0"	66	
Coping on Abutment	2x4	19"	19"	0'4"	24	
Parapet walls	2x2	3'6"	1'3"	1'4"	6	
					134	150
7 (a) Reinforced concrete 1:2:4 with 3/4" hard broken stone						
(a) For deckslab	1	98'0"	13'6"	2'1"	27563	
kerbs	2	98'0"	0'9"	0'11"	1347.8	
(b) Kerbs					2911.13	2911
Wingwalls	2x27	0'5"	0'7"	3'6"	25.52	
	2x2	98'0"	0'5"	0'6"	80.83	
					106.35	107
8. Reinforced Concrete 1:1½:3 mixed with 66⅔% 1" gauge stone shingle and 33⅓% 3/4" hard broken stone						
For coping cast	1	101'6"	1'0"	0'3"	303	310
9 Reinforced Cement for expansion joints						
Footpath	2x2	15'0"	2'0"	2'0"	240	
Wingwalls	1x2	10'0"	2'0"	2'0"	160	
Deckslab	2x2	100'0"	1'0"	1'0"	400	
Wingwalls	3x2	10'0"	1'0"	1'0"	400	
					1200	1300
10. Extra for [illegible]						
11 Cast iron [illegible]					2.5	
12 Petty & provisional Charges					2.5	

27.12

Divl. Engineer (H),
Community Projects,
Mangalore.

Recd 13/2

Superintending
7.12.54 J.E. Hoskin

P.W.D. V-13—67,416— 20-6-52.

ESTIMATE FORM—Abstract.

Item number. (1)	Quantity. (2)	Description of work. (3)	Rate. (4)	Per. (5)	Amount. (6)	Total. (7)
			Rs.		Rs.	Rs.
	10 cft	Earthwork in all soils except rock ~~and~~ in Library	21-60	100 cft	214	
	500 sft	Bench work in 2 lays 2" thick 1:3:6	2-80 ~~3-120~~ ~~3-120~~	sft	625 ~~625~~ 620/-	
	525 cft	Cement Concrete ~~1:4:8~~ 1:3:6 with 1½" hard broken stones	118-00	100 cft	~~630~~	
	3950 cft ~~4100 cft~~	Coursed Rubble Masonry II sort in Cement mortar 1:5	94-00	100 cft	3713/- 3854	
	1400	Coursed Rubble masonry I sort in C.M. 1:5	115-00	100 cft	1610	
	2891 cft	Reinforced Cement Concrete 1:2:4 with ¾" hard broken stones for leads by — do — Land rail transport	4-80	cft	13010	
	107		13-00	cft	1391	
	150	Cement Concrete 1:2:4 with ¾" hard broken stones for leads by	177-00	100 cft	266	
	310	Cement Concrete 1:1½:3 mix with 66⅔% of 1½" size hard broken stones and 33⅓% of ¾" hard broken stones	208-00	100 cft	645	
	1300	Rough stone dry pitching for approaches	33-40	100 cft	432	
	L.S.	Earthwork in hard gravelly soil for forming approach road			~~1100~~ to	
	L.S.	~~Earthwork~~ Contingencies and unforeseen items including water, constructional plant ~~charges~~			~~1111~~ 1249/-	
	L.S.	Petty supervision charges			~~300~~/- ~~24000~~/-	25000/-
		Rupees Twenty five thousand only				

Technical sanction accorded for Rupees Twenty five thousand only by the DRHQ CP/54-55

M. Khanna
Divl. Engineer (H)
Community Projects,
Mangalore.

Manager
KEL
13/12

Comm. Projects
12/1/55

Director of Community Development
12-1-55 12/1/55 16-1-55

Pub(RDP VI)

R

Proceedings of the D. C. D. No. 151 Pub(RDP VI) 55-1 dt. 20-1-55

Sub:- Cls. S. Kanara Area. ~~Block II~~ 1955-56

Communications. Construction of a bridge on the road from Venur to Munjea - Estimate Sanctioned.

plep

Ref:- From the PEO. Mangalore Ref. C. No. 3297/53-B dated 26-12-54

No. 151 Pub(RDP VI) 55-1 dt. 20-1-55

Order:-

Sanction is accorded to an expenditure not exceeding Rs 25,000/- (Rupees Twenty five thousand only) being the estimated cost of Constructing a bridge on the kutcha road from Venur to Munjea. The kutcha road 5½ miles long and will be formed by the beneficiaries together with the Construction of necessary paved dips etc at an approximate cost of Rs 12900/- and this is deemed as their Contribution. This sanction is Counted against the provision in item IH. Communications in the budget estimates of this Area approved by the C.A. in their letter No. CA 558/55/53 (Bud) dated 6-7-54.

2. The expenditure should be debited to "63 B. C.D. Projects - J. Communi - Cations - 1. Works"

3. The PEO. Mangalore is requested to see that the work is got executed speedily

ns p 20

eeru dtd 16.1.55 estimate and plan approved by the Spl D.E.(H) which are herewith returned Countersigned

DT
16.1.55
12/1/55

31/1/55

To

The PEO. Mangalore (H&E). (by regd post)
The AG. Madras
Copy to The Secy. C.P.A. Regal Bldgs. New Delhi.
The Collector of S. Kanara
Pub R&B VII, VIII & Tech.

SF. 1111

CENTRAL DESPATCH BRANCH
Van.
Despatched by Foot
Reg. Post.
Messenger.
195 . Despatcher's Initials.

O.N :-

Please see the letter from the PEO. Mangalore at p/cy. The estimate for Rs 25000/- for the bridge ~~is~~ has requires to be sanctioned entirely ~~to be met~~ from C.P. funds. for the reason that the formation of the road is to be borne by the beneficiaries. The road is reported to be included in the Approved programme and the cost of the bridge is to be met against the provision of Rs 10.55 lakhs for bridges and culverts included in the budget estimates approved by the C.P.A.

The draft proceedings above may be Approved
The estimate may also be Countersigned

2 Copies kept below.

DT
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